

Elasto-dynamics with plasticity and contact in the nonsmooth framework.

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The logo for Inria, featuring the word "Inria" in a stylized, red, cursive script.

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The logo for Université Grenoble Alpes, featuring the letters "UGA" in a bold, blue, sans-serif font, with a small orange triangle below the "A". Below "UGA" is the text "Université Grenoble Alpes" in a smaller, blue, sans-serif font.

Motivations and Objectives

Elasto-dynamics with plasticity and unilateral contact

1. A single differential variational inequality
 - ▶ pioneering work of J.J. Moreau (1974), Halphen & Nguyen (1975)
 - ▶ multi-criteria elasto-plastic flow rules with contact constraints.
2. A nonsmooth dynamical framework for finite-dimensional systems
 - ▶ dealing with jumps in velocities and impulsive forces (FEM discretized)
3. A Moreau-Jean type time-stepping method,
 - ▶ enabling the consistent integration of the nonsmooth dynamics.
4. A discrete energy balance
 - ▶ a practically stable scheme with positive dissipation.
5. Variational approach:
 - ▶ formulation of a saddle point problem (min-max problem) and a convex quadratic program
 - ▶ well-posedness results (existence and uniqueness)
 - ▶ numerical optimization methods as an alternative to return-mapping algorithm

Formulation of elastodynamics with contact

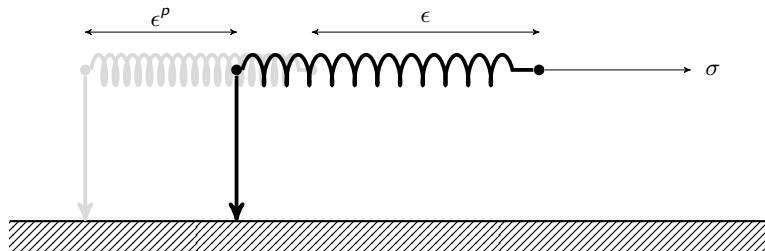
Space and time discretization

Well posedness of the discrete problem

Discrete energy balance

Conclusion and perspectives

1D plasticity



Perfect 1D plasticity

$$\begin{cases} \sigma(t) = E(\epsilon(t) - \epsilon^P(t)) \\ \dot{\epsilon}^P(t) \in N_C(\sigma(t)) \end{cases} \quad (1)$$

with C the initial elastic domain (when ϵ^P is equal to zero) given by

$$C = [-\sigma_c, \sigma_c] \quad (2)$$

Perfect 1D plasticity with linear kinematic hardening

$$\begin{cases} a(t) = D\epsilon^P(t) \\ \sigma(t) = E(\epsilon(t) - \epsilon^P(t)) \\ \tau(t) = \sigma(t) - a(t) \\ \dot{\epsilon}^P(t) \in N_C(\tau(t)) \end{cases} \quad (3)$$

$a(t)$ is the hardening force.

1D plasticity

Let us study the model assuming that the strain is a given function of time $e(t)$, that is

$$\epsilon(t) = e(t) \quad (4)$$

Substituting to keep only $\epsilon^P(t)$ we get the following differential inclusion

$$\begin{cases} \dot{\epsilon}^P(t) \in N_C(\tau(t)) \\ \tau(t) = Ee(t) - (E + D)\epsilon^P(t), \end{cases} \quad (5)$$

or, equivalently

$$\dot{\epsilon}^P(t) \in N_C(Ee(t) - (E + D)\epsilon^P(t)). \quad (6)$$

1D plasticity

To get a standard form for the inclusion, we may prefer to use $\tau(t)$ as the state variable. We obtain

$$\dot{e}^p(t) = \frac{E\dot{e}(t) - \dot{\tau}(t)}{E + D} \quad (7)$$

and

$$-\dot{\tau}(t) + Ee(t) \in N_C(\tau(t)) \quad (8)$$

which is clearly a maximal monotone differential inclusion. We have existence and uniqueness of the absolutely continuous solutions for $e(t)$ Lipschitz continuous [Moreau (1976)]

In the particular case of a polyhedral set C , the differential inclusion is equivalent to a linear complementarity systems of relative degree 1. For analytical solution, we need to differentiate the complementarity condition, often called consistency conditions.

Plasticity in the generalized standard materials framework

Simplest framework: Small perturbation, associative plasticity and linear hardening

- ▶ Small perturbations hypothesis with additive decomposition of the strain

$$\boldsymbol{\varepsilon} = \boldsymbol{\varepsilon}^e + \boldsymbol{\varepsilon}^p. \quad (9)$$

- ▶ Linear elasticity and hardening laws

$$\boldsymbol{\sigma} = \mathbf{E} : \boldsymbol{\varepsilon}^e \quad \text{and} \quad \mathbf{a} = -\mathbf{D} \cdot \boldsymbol{\alpha}. \quad (10)$$

- ▶ Generalized standard material (GSM) (associative plasticity)

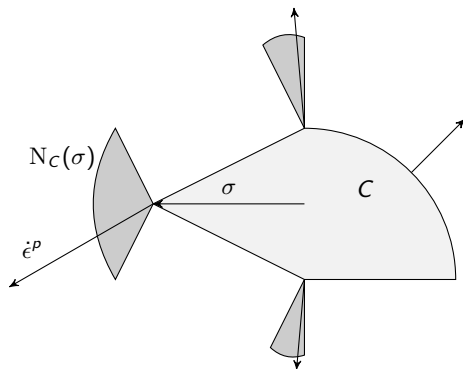
$$\begin{pmatrix} \dot{\boldsymbol{\varepsilon}}^p \\ \dot{\boldsymbol{\alpha}} \end{pmatrix} \in \text{Nc} \begin{pmatrix} \boldsymbol{\sigma} \\ \mathbf{a} \end{pmatrix}. \quad (11)$$

$\mathbf{C}(\boldsymbol{\sigma}, \mathbf{a})$ a convex set of admissible stresses $\boldsymbol{\sigma}$ and hardening forces \mathbf{a}

- ▶ Clausius-Duhem dissipation inequality is automatically satisfied

$$d = \boldsymbol{\sigma} : \dot{\boldsymbol{\varepsilon}}^p + \mathbf{a} \cdot \dot{\boldsymbol{\alpha}} \geq 0 \quad \text{if} \quad 0 \in \mathbf{C}. \quad (12)$$

Plasticity in the generalized standard materials framework



$$\begin{pmatrix} \dot{\epsilon}^P \\ \dot{\alpha} \end{pmatrix} \in N_C \begin{pmatrix} \sigma \\ a \end{pmatrix}$$

 \Leftrightarrow

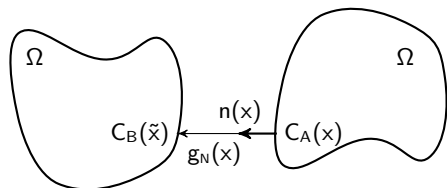
$$C = \{(\sigma, a), f(\sigma, a) \geq 0\}$$

$$\begin{cases} \dot{\epsilon}^P = -\nabla_{\sigma}^T f(\sigma, a) \lambda \\ \dot{\alpha} = -\nabla_a^T f(\sigma, a) \lambda \\ 0 \leq \lambda \perp f(\sigma, a) \geq 0, \end{cases}$$

$$\begin{pmatrix} \sigma \\ a \end{pmatrix} = \text{proj}_C \left(\begin{pmatrix} \sigma \\ a \end{pmatrix} - \rho \begin{pmatrix} \dot{\epsilon}^P \\ \dot{\alpha} \end{pmatrix} \right), \rho > 0$$

(13)

Unilateral contact



$g_N(x)$: gap function

$v_N(x) = \frac{d}{dt}g_N(x)$: relative velocity

- ▶ Signorini contact law:

$$0 \leq g_N \perp r_N \geq 0 \iff -r_N \in N_{\mathbb{R}_+}(g_N). \quad (14)$$

- ▶ Signorini contact law at the velocity level:

$$\begin{aligned} 0 \leq r_N \perp v_N \geq 0 \text{ if } g_N = 0, \text{ else } r_N = 0 \\ \Downarrow \\ -r_N \in N_{T_{\mathbb{R}_+}(g_N)}(v_N), \end{aligned} \quad (15)$$

where $T_{\mathbb{R}_+}(g_N)$ is the tangent cone of \mathbb{R}_+ at g_N

Space discretization

Standard FEM discretization to make is as simple as possible.

Finite dimensional smooth linear dynamics:

$$M\dot{v}(t) + B^T\sigma(t) = f_{\text{ext}}(t) + r(t). \quad (16)$$

- ▶ u, v nodal displacement and velocity vector,
- ▶ σ stress at Gauss points,
- ▶ M constant mass matrix,
- ▶ B^T discrete divergence operator, B is the discrete gradient.

Elasto-plastic relation at Gauss points and contact kinematics

$$\left\{ \begin{array}{l} \sigma = E\varepsilon^e = E(\varepsilon - \varepsilon^p) \\ a = -D\alpha \\ \begin{pmatrix} \dot{\varepsilon}^p \\ \dot{\alpha} \end{pmatrix} \in N_C \begin{pmatrix} \sigma \\ a \end{pmatrix}, \end{array} \right. \quad (17)$$

$$\begin{array}{l} v_N = H^T(u)v \\ r = H(u)r_N. \end{array} \quad (18)$$

Measure differential equation

Ability to deal with velocity jumps and impulsive forces in discrete systems

$$Mdv + B^\top \sigma(t)dt = f_{\text{ext}}(t)dt + H(u(t))di_N. \quad (19)$$

- ▶ dv differential measure (“acceleration as measure”)
- ▶ di_N contact reaction measure

Unilateral contact and Newton impact law

$$- di_N \in N_{T_{\mathbb{R}_+^m}(\mathcal{g}_N(t))}(v_N(t) + e v_N^-(t)). \quad (20)$$

where e is the coefficient of restitution

Standard FEM discretization

After the differentiation of the constitutive law with $S = E^{-1}$ (similar to incremental formulation) and the introduction of slack variable

$$y = -\dot{\alpha} \text{ and } z = -\dot{\epsilon}^p$$

we get a differential measure variational inequality

$$\left\{ \begin{array}{l} Mdv + B^\top \sigma(t)dt = f_{\text{ext}}(t)dt + H(u(t))di_N \\ \dot{u}(t) = v(t) \\ S\dot{\sigma}(t) = Bv(t) + z(t) \\ D^{-1}\dot{a}(t) = y(t) \\ v_N(t) = H^\top(u(t))v(t) \\ - \begin{pmatrix} z(t) \\ y(t) \\ di_N \end{pmatrix} \in N_{C \times T_{\mathbb{R}_+^m}(g_N(t))} \begin{pmatrix} \sigma(t) \\ a(t) \\ (v_N(t) + \epsilon v_N^-(t)) \end{pmatrix} \end{array} \right. \quad (21)$$

A **single** variational inequality.

Time stepping scheme

Extension of the Moreau–Jean scheme with $\theta \in (0, 1]$

$$\left\{ \begin{array}{l} M(v_{k+1} - v_k) + hB^\top \sigma_{k+\theta} = hf_{\text{ext},k+\theta} + Hp_{N,k+1} \\ S(\sigma_{k+1} - \sigma_k) - hBv_{k+\theta} = hz_{k+\theta}, \\ D^{-1}(a_{k+1} - a_k) = hy_{k+\theta} \\ v_{N,k+1} = H^\top v_{k+1} \\ - \begin{pmatrix} z_{k+\theta} \\ y_{k+\theta} \\ p_{N,k+1} \end{pmatrix} \in N_{C \times \mathbb{R}_+^m} \left(\begin{pmatrix} \sigma_{k+\theta} \\ a_{k+\theta} \\ v_{N,k+1} + ev_{N,k} \end{pmatrix} \right) \end{array} \right. \quad (22)$$

where

- ▶ notation: h time step, $x_{k+\theta} = \theta x_{k+1} + (1 - \theta)x_k$
- ▶ the Signorini condition only on the active contact ($g_{N,k} \leq 0$)
- ▶ displacements are updated afterwards $u_{k+1} = u_k + hv_{k+\theta}$
- ▶ impulses $p_{N,k+1}$ as primary variable
- ▶ elasto-plastic law with hardening is solved at $k + \theta$.

Variational approach: saddle point problem

Proposition (Saddle point problem)

The solutions of $(v, \dot{\varepsilon}, \sigma, a, v_N)$ of the first order optimality conditions of

$$\begin{aligned} \min_{v, \dot{\varepsilon}} \max_{\sigma, a} \quad & \frac{1}{2}(v - v_k)^\top M(v - v_k) \\ & - \frac{1}{2}(\sigma - \sigma_k)^\top S(\sigma - \sigma_k) - \frac{1}{2}(a - a_k)^\top D^{-1}(a - a_k) \\ & + h\theta\sigma^\top \dot{\varepsilon} - h\theta f_{\text{ext}, k+1}^\top v \end{aligned}$$

s. t.

$$\begin{aligned} Bv &= \dot{\varepsilon} \\ \theta v_N &= H^\top v - (1 - \theta)v_{N,k} \\ \begin{pmatrix} \sigma \\ a \\ v_N + ev_{N,k} \end{pmatrix} &\in C \times \mathbb{R}_+^m. \end{aligned}$$

(23)

are solutions of (22) for $(v_{k+\theta}, z_{k+\theta}, \sigma_{k+\theta}, a_{k+\theta}, v_{N, k+1})$.

- ▶ A kind of discrete ∂' Alembert principle for elastoplasticity with contact.

Variational approach: saddle point problem

Optimality conditions

Consider the Lagrangian function

$$\begin{aligned}\mathcal{L}(v, \dot{\varepsilon}, v_N, \sigma, a, \lambda, \mu) &= \frac{1}{2}(v - v_k)^\top M(v - v_k) \\ &\quad - \frac{1}{2}(\sigma - \sigma_k)^\top S(\sigma - \sigma_k) - \frac{1}{2}(a - a_k)^\top D^{-1}(a - a_k) \\ &\quad + h\theta\sigma^\top \dot{\varepsilon} - h\theta f_{\text{ext},k+1}^\top v \\ &\quad + \lambda^\top (Bv - \dot{\varepsilon}) + \mu^\top (\theta v_N - H^\top v + (1 - \theta)v_{N,k}) \\ &\quad - h\theta\Psi_C \begin{pmatrix} \sigma \\ a \end{pmatrix} + \theta^2\Psi_{\mathbb{R}_+^m}(v_N + ev_{N,k}).\end{aligned}\tag{24}$$

Variational approach: saddle point problem

Optimality conditions

$$\begin{aligned}(\nabla_v \mathcal{L} :) \quad 0 &= M(v - v_k) - h\theta f_{\text{ext},k+1}^\top + B^\top \lambda - H\mu \\(\nabla_{\dot{\varepsilon}} \mathcal{L} :) \quad 0 &= h\theta\sigma - \lambda \\(\partial_\sigma \mathcal{L} :) \quad 0 &\in -S(\sigma - \sigma_k) + h\theta\dot{\varepsilon} - h\theta\partial_\sigma \Psi_C \begin{pmatrix} \sigma \\ a \end{pmatrix} \\(\partial_a \mathcal{L} :) \quad 0 &\in -D^{-1}(a - a_k) - h\theta\partial_a \Psi_C \begin{pmatrix} \sigma \\ a \end{pmatrix} \\(\partial_{v_N} \mathcal{L} :) \quad 0 &\in \theta\mu + \theta^2\partial\Psi(v_N + ev_{N,k}) \\(\nabla_\lambda \mathcal{L} :) \quad 0 &= Bv - \dot{\varepsilon} \\(\nabla_\mu \mathcal{L} :) \quad 0 &= \theta v_N - H^\top v + (1 - \theta)v_{N,k}\end{aligned} \tag{25}$$

Note that $h\theta\sigma$ appears as the Lagrange multiplier that enforces the condition $Bv = \dot{\varepsilon}$.

Variational approach: saddle point problem

Optimality conditions

Introducing the variables z, y, p_N such that

$$-\begin{pmatrix} z \\ y \\ p_N \end{pmatrix} \in \partial \Psi_{C \times \mathbb{R}_+^m} \left(\begin{pmatrix} \sigma \\ a \\ v_N + e v_{N,k} \end{pmatrix} \right), \quad (26)$$

and simplifying the equations, we obtain

$$\left\{ \begin{array}{l} M(v - v_k) + h\theta B^\top \sigma = h\theta f_{\text{ext},k+1}^\top + \theta H p_N \\ S(\sigma - \sigma_k) - h\theta B v = h\theta z \\ D^{-1}(a - a_k) = h\theta y \\ \theta v_N = H^\top v - (1 - \theta)v_{N,k} \\ -\begin{pmatrix} z \\ y \\ p_N \end{pmatrix} \in \partial \Psi_{C \times \mathbb{R}_+^m} \left(\begin{pmatrix} \sigma \\ a \\ v_N + e v_{N,k} \end{pmatrix} \right) \end{array} \right. \quad (27)$$

Variational approach: saddle point problem

Assumption (1)

The matrices M , S and D are symmetric definite positive matrices.

Assumption (2)

It exists v^0, σ^0, a^0 such that

$$\left\{ \begin{array}{l} \begin{array}{l} \left(\begin{array}{l} \sigma^0 \\ a^0 \end{array} \right) \in C \\ \\ H^\top v^0 + (\theta(1 + e) - 1)v_{N,k} \geq 0 \end{array} \right. \quad \begin{array}{l} 0 \text{ is at least in } C \\ \\ \text{standard feasibility condition} \end{array} \end{array} \quad (28)$$

Proposition

Under Assumptions 1 and 2, the saddle-point problem (23) has a unique solution $(v, \dot{\varepsilon}, \sigma, a, v_N)$.

Variational approach: convex quadratic problem

Substitution of v_{k+1} in the linear equations in (22)

Reduction to local variables.

$$-\left(Q \begin{pmatrix} \sigma_{k+\theta} \\ \mathbf{a}_{k+\theta} \\ \mathbf{p}_{N,k+1} \end{pmatrix} + \mathbf{p} \right) \in N_{C \times \mathbb{R}_+^m} \begin{pmatrix} \sigma_{k+\theta} \\ \mathbf{a}_{k+\theta} \\ \mathbf{p}_{N,k+1} \end{pmatrix}, \quad (29)$$

with

$$Q = \begin{pmatrix} U & 0 & -V \\ 0 & D^{-1} & 0 \\ -V^\top & 0 & W \end{pmatrix} \text{ and } \mathbf{p} = \begin{pmatrix} s \\ D^{-1} \mathbf{a}_k \\ r \end{pmatrix}. \quad (30)$$

and

$$\begin{aligned} W &= \theta^2 H^\top M^{-1} H && \text{Delassus matrix} \\ U &= S + h^2 \theta^2 B M^{-1} B^\top \\ V^\top &= h \theta^2 H^\top M^{-1} B^\top \\ s &= -S \sigma_k - h \theta B (v_k + \theta h M^{-1} f_{\text{ext},k+\theta}) \\ r &= \theta^2 \left(e v_{N,k} + H \left(v_k + \theta M^{-1} (h f_{\text{ext},k+\theta}) \right) \right) \end{aligned} \quad (31)$$

Variational approach: convex quadratic problem

Equivalent convex minimization problem:

Equivalent convex minimization problem:

$$\begin{aligned} \min_{\sigma, a, p_N} \quad & \frac{1}{2} \begin{pmatrix} \sigma \\ a \\ p_N \end{pmatrix}^\top Q \begin{pmatrix} \sigma \\ a \\ p_N \end{pmatrix} + p^\top \begin{pmatrix} \sigma \\ a \\ p_N \end{pmatrix} \\ \text{s.t.} \quad & \begin{pmatrix} \sigma \\ a \\ p_N \end{pmatrix} \in C \times \mathbb{R}_+^m. \end{aligned} \tag{32}$$

Assumption (3)

The matrix H has full rank.

Proposition

Under Assumptions 1 and 3 and for a sufficiently small time step, the problem (32) has a unique solution (σ, a, p_N) if the set $0 \in C$.

Convex Quadratic problem

C finitely represented

$$C = \{(\sigma, a) \mid f(\sigma, a) \geq 0\}, \quad (33)$$

f is a smooth vector-valued function with **non-vanishing gradients**

$$\begin{aligned} \min_{\sigma, a, p_N} \quad & \frac{1}{2} \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix}^\top Q \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} + p^\top \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} \\ \text{s.t.} \quad & f(\sigma, a) \geq 0 \\ & p_N \geq 0 \end{aligned} \quad (34)$$

The case of J_2 perfect plasticity.

The J_2 plasticity is based on the Huber-Von Mises criterion amounts to defining the yield function f with the J_2 invariant of the stress $\boldsymbol{\sigma}$.

$$J_2 = \frac{1}{2} \mathbf{s} : \mathbf{s} = \frac{1}{2} s_{ij} s_{ji} = \frac{1}{2} \text{tr}(\mathbf{s} \cdot \mathbf{s}) \quad (35)$$

where \mathbf{s} is the deviatoric part of the tensor $\boldsymbol{\sigma}$, and the hydrostatic stress σ^h defined by

$$\mathbf{s} = \boldsymbol{\sigma} - \sigma^h \mathbf{1} = \boldsymbol{\sigma} - \frac{1}{3} \text{tr}(\boldsymbol{\sigma}) \mathbf{1} = \boldsymbol{\sigma} - \frac{1}{3} h_1 \mathbf{1} \quad \text{with } \sigma^h = \frac{1}{3} h_1, h_1 = \boldsymbol{\sigma} : \mathbf{1} = \sigma_{ii} = \text{tr}(\boldsymbol{\sigma}). \quad (36)$$

The Huber-Von Mises criterion is

$$\sqrt{3J_2} \leq \kappa, \text{ or } 3J_2 \leq \kappa^2. \quad (37)$$

The case of J_2 perfect plasticity.

With an alternative expression of the J_2 invariant, another expression of the Huber-Von Mises criterion is in the original stress tensor:

$$\frac{1}{2} \left((\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{12}^2 + \sigma_{13}^2 + \sigma_{23}^2) \right) \leq \kappa^2. \quad (38)$$

Using the Voigt notation for the stress

$\sigma = \left(\sigma_{11} \quad \sigma_{22} \quad \sigma_{33} \quad \sigma_{23} \quad \sigma_{13} \quad \sigma_{12} \right)^\top$, the J_2 invariant can be expressed as a quadratic function

$$s^\top s = \sigma^\top J \sigma, \quad (39)$$

with

$$J = \frac{1}{3} \begin{pmatrix} 2 & -1 & -1 & 0 & 0 & 0 \\ -1 & 2 & -1 & 0 & 0 & 0 \\ -1 & -1 & 2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 6 & 0 & 0 \\ 0 & 0 & 0 & 0 & 6 & 0 \\ 0 & 0 & 0 & 0 & 0 & 6 \end{pmatrix}. \quad (40)$$

The matrix J is semi-definite positive which shows that C is convex.

The case of J_2 perfect plasticity.

In that case, we have to solve a quadratic program with quadratic constraints that can be solved efficiently with SQP solvers

$$\begin{aligned} \min_{\sigma, a, p_N} \quad & \frac{1}{2} \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix}^\top Q \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} + p^\top \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} \\ \text{s.t.} \quad & 3\sigma_g^\top J\sigma_g \geq \kappa^2 \text{ for } g \in \llbracket 1, n_{e,g} \rrbracket \\ & p_N \geq 0. \end{aligned} \tag{41}$$

The case of Drucker-Prager plasticity.

The Drucker-Prager plasticity involves the following definition of the convex set C as

$$C := K_\sigma = \left\{ (\sigma^h, \mathbf{s}) \mid \frac{1}{k_d} \|\mathbf{s}\| + \sigma^h \tan(\varphi) \leq c \right\} \quad (42)$$

where $\|\mathbf{s}\| = \sqrt{J_2}$, k_d is a constant that depends on the dimension d , c is the cohesion and φ the friction angle.

The set K_σ is a translated second order cone

- ▶ it cannot be straightforwardly written with a smooth function f as in (33)
- ▶ it can be formulated on symmetric cone of semi-definite type or second order types [Berga, De Saxcé(1994), Hjjaj et al. (2203), Bisbos et al. (2005)]]
- ▶ Tools from second-order cone or semi-definite programming can be used

The case of Drucker-Prager plasticity.

$$\begin{aligned} \min_{\sigma, a, p_N} \quad & \frac{1}{2} \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix}^\top Q \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} + p^\top \begin{pmatrix} \sigma \\ p_N \\ a \end{pmatrix} \\ \text{s.t.} \quad & \sigma^h = \frac{1}{3} \text{tr}(\sigma), \\ & s = \sigma - \sigma^h I, \\ & (\sigma^h, s)_g \in K_\sigma \text{ for } g \in \llbracket 1, n_{e,g} \rrbracket \\ & p_N \geq 0. \end{aligned} \tag{43}$$

The case of Drucker-Prager plasticity.

Associative plasticity

The optimality conditions related to the constraints $(\sigma^h, s)_g \in K_\sigma$ defines the following flow rule

$$\begin{pmatrix} \dot{\varepsilon}^{p,h} \\ \dot{e}^p \end{pmatrix} \in N_{K_\sigma} \begin{pmatrix} \sigma^h \\ s \end{pmatrix}, \quad (44)$$

where $e = \varepsilon - \frac{1}{3}\varepsilon^h I$ and $\varepsilon^h = \text{tr } \varepsilon$.

Non associative plasticity [Berga De Saxcé (1994)]

$$\begin{pmatrix} \dot{\varepsilon}^{p,h} + k_d(\tan(\varphi) - \tan(\theta))\|\dot{e}^p\| \\ \dot{e}^p \end{pmatrix} \in N_{K_\sigma} \begin{pmatrix} \sigma^h \\ s \end{pmatrix}, \quad (45)$$

where θ is the dilatancy angle ranging from 0 to φ .
no direct associated optimisation problem.

Numerical methods from optimization

- ▶ Direct pivoting techniques (Active set, Lemke, PATH)
Direct solution if C is polyhedral
- ▶ First order (Nesterov) accelerated techniques (ADMM, APGD, ...)
- ▶ Second order methods:
 1. interior point methods
 2. semi-smooth Newton method

$$0 = \begin{pmatrix} \sigma \\ a \\ \rho_N \end{pmatrix} - \text{proj}_{C \times \mathbb{R}_+^m} \left[\begin{pmatrix} \sigma \\ a \\ \rho_N \end{pmatrix} - \rho \left(Q \begin{pmatrix} \sigma \\ a \\ \rho_N \end{pmatrix} + p \right) \right], \quad \rho > 0$$

Comments on semi-smooth Newton methods

Similarity with return-mapping algorithm [Hager, Wolhuth (2009), Christensen(2009)]
but with more flexibility:

- ▶ optimal choice of ρ with self-adaptation (proximal technique)
- ▶ rescaling technique are easier
- ▶ no explicit need of a consistent tangent operator

Discrete energy balance

- ▶ Energy balance for discrete systems:

$$dE(t) = d(T(t) + \Psi(t)) = -D(t)dt + P_{\text{ext}}(t)dt + dP_{\text{impact}}, \quad (46)$$

with

$$T(t) = \frac{1}{2}v^\top(t)Mv(t), \quad \text{and} \quad \Psi(t) = \frac{1}{2}\varepsilon^e{}^\top(t)E\varepsilon^e(t) + \frac{1}{2}\alpha^\top(t)D\alpha(t) \quad (47)$$

- ▶ The dissipation and the power of external forces are

$$D(t) = \sigma^\top(t)\dot{\varepsilon}^p(t) + a^\top(t)\dot{\alpha}(t) \quad \text{and} \quad P_{\text{ext}}(t) = f_{\text{ext}}^\top(t)v(t). \quad (48)$$

- ▶ The power of the reaction impulse is given by

$$dP_{\text{impact}} = \frac{1}{2}(v_N^+ + v_N^-)d\dot{N}. \quad (49)$$

Discrete energy balance

Approximation of works by the θ -method as

$$W_{\text{ext } k}^{k+1} := h \mathbf{v}_{k+\theta}^\top \mathbf{f}_{\text{ext}, k+\theta} \approx \int_{t_k}^{t_{k+1}} P_{\text{ext}}(t) dt, \quad (50)$$

and

$$W_p^{k+1} := h \sigma_{k+\theta}^\top \dot{\varepsilon}_{k+\theta}^p - h \mathbf{a}_{k+\theta}^\top \mathbf{y}_{k+\theta} \approx \int_{t_k}^{t_{k+1}} D(t) dt, \quad (51)$$

Approximation of the work dissipated by the percussion

$$W_c^{k+1} := \mathbf{v}_{N, k+\theta}^\top \mathbf{p}_{N, k+1} = (1 - \theta(1 + e)) \mathbf{v}_{N, k}^\top \mathbf{p}_{N, k+1}, \quad (52)$$

the increment of total energy is then given by

$$\Delta E_k^{k+1} - W_{\text{ext } k}^{k+1} + W_p^{k+1} - W_c^{k+1} = \left(\frac{1}{2} - \theta\right) \left(\|\mathbf{v}_{k+1} - \mathbf{v}_k\|_M^2 + \|\varepsilon_{k+1}^e - \varepsilon_k^e\|_E^2 + \|\alpha_{k+1} - \alpha_k\|_D^2 \right). \quad (53)$$

Proposition

Under Assumption 1, energy dissipation of the scheme is as follows

1. When $\theta = \frac{1}{2}$, the time-stepping scheme satisfies the approximation of the discrete energy balance :

$$\Delta E_k^{k+1} - W_{\text{ext } k}^{k+1} = -W_{p \ k}^{k+1} + W_{c \ k}^{k+1}. \quad (54)$$

2. The dissipated work due to plasticity is *always* positive
3. When $\theta \leq \frac{1}{1+e}$, the dissipated work due to impact is also positive.
4. When $\frac{1}{2} \leq \theta \leq \frac{1}{1+e} \leq 1$, we have the following dissipation inequality

$$\Delta E_k^{k+1} - W_{\text{ext } k}^{k+1} \leq 0. \quad (55)$$

Conclusions & perspectives

Conclusions

A monolithic solver for elastodynamics with contact, impact and plasticity:

- ▶ A practically stable scheme with a discrete energy balance
- ▶ A variational formulation (optimization problem) of plasticity with contact
- ▶ A gateway to a host of multiple optimisation algorithms
- ▶ Useful also for quasi-static application, even with perfect plasticity

Vincent Acary, Franck Bourrier, Benoit Viano.

Variational approach for nonsmooth elasto-plastic dynamics with contact and impacts. 2023. [\(hal-03978387v1\)](#) to appear in Computer Methods in Applied Mechanics and Engineering

Perspectives

- ▶ Non associated plasticity and Coulomb friction
 - ▶ Implicit standard materials (De Saxcé) → quasi-variational inequality
- ▶ Finite strain plasticity and mortar method ([Seitz, Popp, Wall (2015)])
- ▶ Material point method (PhD Louis Guillet)
 - ▶ Application to gravity-driven flows of geomaterials in mountains (mud and debris flows).



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